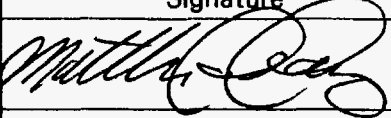

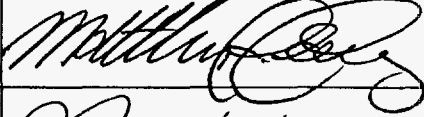



Design Analysis Cover Sheet

Complete only applicable items.

①

WBS: 1.2.6
QA: QA N/A 03/28/95
Page: 1 Of: 9

2. DESIGN ANALYSIS TITLE DIESEL FUEL TANK FOUNDATIONS		MOL.19951101.0035	
3. DOCUMENT IDENTIFIER BABBDA000-01717-0200-00007 REV 00		4. REV. NO. 00	5. TOTAL PAGES 9
6. TOTAL ATTACHMENTS/NO. OF PAGES IN EACH NONE		7. SYSTEM ELEMENT ESF	
	Print Name	Signature	Date
8. Originator	M. Gomez		01/17/95
9. Checker	J. Salchak		01/18/95
10. Lead Discipline Engineer	M. Gomez		01/18/95
11. Department Manager	J. Salchak		01/18/95
12. REMARKS			

Design Analysis Revision Record

Complete only applicable items.

①

WBS:

QA:

Page:

1.2.6.

2/10/50

2 Of: 9

[illegible]

1. PURPOSE

The purpose of this analysis is to design structural foundations for the Diesel Fuel Tank and Fuel Pumps. This analysis is in support of design drawing BABBDF000-01717-2100-23082.

2. QUALITY ASSURANCE

The items considered within this analysis relate to temporary equipment foundations not included on the Q-list. There are no Q-Controls associated with this analysis.

3. METHOD

The equipment foundation shall be designed in Section 10 using standard foundation design hand calculations. The vertical loads will reflect Mechanical requirements. Lateral loads will be calculated using applicable codes. The soil bearing and foundation stresses will be analyzed using accepted engineering mechanics. The foundation will be designed using the Strength Design Method.

4. CODES AND STANDARDS

4.1 U.S. DEPARTMENT OF ENERGY (DOE):

DOE 6430.1A, General Design Criteria
dated April 6, 1989

4.2 AMERICAN CONCRETE INSTITUTE (ACI):

ACI 318-89 Building Code Requirements for Reinforced Concrete

4.3 AMERICAN NATIONAL STANDARDS INSTITUTE, INC./AMERICAN SOCIETY OF CIVIL ENGINEERS (ANSI/ASCE):

ANSI/ASCE 7-88 Minimum Design Loads for Buildings and Other Structures

4.4 UNIFORM BUILDING CODE (UBC):

UBC, 1991

4.5 AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC):

AISC, 9th Edition Manual of Steel Construction, Allowable Stress Design

4.6 AMERICAN WELDING SOCIETY (AWS)

AWS D1.1-94

Structural Welding Code-Steel

5. DESIGN INPUTS

- 5.1 Exploratory Studies Facility (ESF) Basis for Design (BFD) Document, Package 1D, Section 7.2.4.6 Surface Compressed Air System (BAB000000-01717-6300-00002, Rev. 05)

6. CRITERIA

- 6.1 The Exploratory Studies Facility Design Requirements (ESFDR) (YMP/CM-0019, Rev. 1)
- 6.2 ESF BFD Document, Package 1D (BAB000000-01717-6300-00002, Rev. 05)
- 6.3 Determination of Importance Evaluation for ESF North Portal Pad (BAB000000-01717-0200-00001 Rev. 04)

7. ASSUMPTIONS

None used.

8. REFERENCES

- 8.1 Geotechnical Recommendations for Design, North Ramp Surface Facility, Exploratory Studies Facility, Yucca Mountain Project, Nevada, SCP No. 8.3.1.14.2. (March 19, 1993)

9. COMPUTER PROGRAMS

None used.

10. DESIGN ANALYSIS

- 10.1 This analysis was initially performed under Revision 0, ICN 1 of the ESFDR, with Seismic Zone 4. Revision 0, ICN 2 of the ESFDR revised the lowered criteria to Seismic Zone 3. The initial design is conservative, with foundation sizes based on physical requirements. Therefore, the analysis will reflect the higher criteria of Zone 4.

- 10.2 Allowable Soil Pressure = 2,000 psf

(See Reference 8.1)

10. DESIGN ANALYSIS

10.1 This analysis was initially performed under Revision 0, ICN 1 of the ESFDR, with Seismic Zone 4. Revision 0, ICN 2 of the ESFDR revised the lowered criteria to Seismic Zone 3. The initial design is conservative, with foundation sizes based on physical requirements. Therefore, the analysis will reflect the higher criteria of Zone 4.

10.2 Allowable Soil Pressure = 2,000 psf (See Reference 8.1)

10.3 Passive Soil Pressure = 350 psf per foot of depth (See Reference 8.1)

10.4 Concrete Properties

A. Compressive Strength (f'_c) = 4,000 psi

B. Concrete weight = 150 pcf

10.5 Reinforcing Steel Yield Strength (f_y) = 60 ksi

(Reference pages 6, 7, and 8 for hand calculations)

YUCCA MOUNTAIN SITE CHARACTERIZATION PROJECT

PAGE NO. 6 OF 9

Civilian Radioactive Waste Management System

Management & Operating Contractor

CONTRACT NO. DE-AC01-91RW00134

SUBJECT: DIESEL FUEL TANK FDN

WBS NO: 1.2.6.

DATE: 7-6-94 REV NO: _____CALC NO: 54350-000-0177-0200-00007ORIGINATOR: M. GOMEZ

CHECKED BY: _____

CHECKED DATE: _____

10. DESIGN ANALYSIS (CONTD)10.1 DIESEL FUEL TANK FDN

- HEIGHT EMPTY = 22.5K (NOTE: PUMP LHS. ARE
FULL = 89.5K CONSIDERED
NEGUGABLE)

- SIZE 9'-0" ϕ x 20'-0" LONG

- LATERAL FORCES

A) WIND: $F = q_z G_h C_f A_f$ (ANSI/ASCE 7-88)

$$q_z = 0.0256 (K_z) (I V)^2$$

$$= 15 \text{ psf}$$

$$K_z = 0.8 \quad 0-15'$$

$$I = 1.07 \quad (\text{Cat. III})$$

$$V = 80 \text{ mph}$$

$$G_h = 1.32 \quad < 15'$$

$$C_f = 0.6 \text{ TABLE 12}$$

$$V_D = 29 \text{ ft} = 2.22$$

$$D/\sqrt{A} = 33 > 2.5$$

$$F = 15(1.32)(.6)(A_f)$$

$$F = 11.9 A_f$$

$$A_f = 9'(22) = 198 \text{ ft}^2 \quad (\text{TRANSVERSE})$$

$$(\text{Worst Case})$$

$$F = 11.9(198) = 2360 \text{ \#}$$

B) SEISMIC: $F_p = 2 I C_p W_p$
(UBC) $= 4(1.5)(.75) W_p$
CHAPTER 23) $F_p = .45 W_p$

$$Z = .4 \quad \text{ZONE 4}$$

$$I = 1.5$$

$$C_p = .75 \quad \text{Table 2.5P}$$

$$\text{III.1}$$

$$F_p = .45(89.5K) = 40.3K$$

\therefore SEISMIC GOVERNS

YUCCA MOUNTAIN SITE CHARACTERIZATION PROJECT

PAGE NO. 7 OF 9

Civilian Radioactive Waste Management System

Management & Operating Contractor

CONTRACT NO. DE-AC01-91RW00134

SUBJECT: DIESEL FUEL TANK FDN

WBS NO: 1.2.6.

DATE: 7-6-94 REV NO: _____CALC NO: BABBA000-8717-0200-00007ORIGINATOR: M. GOMEZ

CHECKED BY: _____

CHECKED DATE: _____

- OVERTURNING:

FOUNDATION SIZE = $11' \times 22' \times 1'-8"$

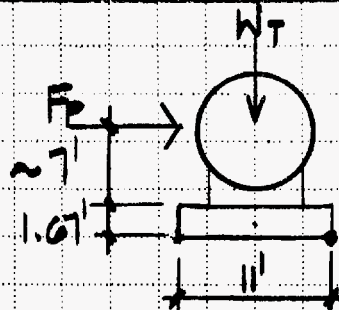
$$F_P = 40.3K$$

$$W_T = 89.5K + 11'(22' \times 1.67')(1.5) = 150.1K$$

$$M_{OT} = 40.3K(9') = 363K'$$

$$M_R = 150.1K(1\frac{1}{2}') = 826K'$$

$$STABILITY RATIO = 826/363 = 2.3 \quad \underline{OK}$$



- SOIL BEARING:

$$e = M/P = 345/145 = 2.38' > \frac{1}{6} = 1.83'$$

$$S.B. = \frac{P}{A} \left[\frac{4L}{3L - 6e} \right] = \frac{150.1K}{11'(22')} \left[\frac{4(11)}{3(11) - 6(2.38)} \right]$$

$$S.B. = 1.46 KSF < 1.35(2) = 2.67 KSF \quad \underline{OK}$$

- REINFORCING: DESIGN FOR MAXIMUM SUPPORT WIDTH

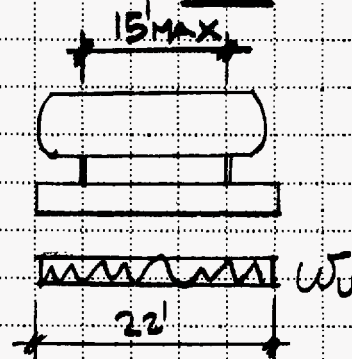
$$w_{u,max} = 1.0(7)(.75)(1.40) = 2.105$$

$$M_U = 2.105 \left[\frac{15^2}{8} - \frac{3.5^2}{2}(2) \right] = 32.5K'$$

$$K_U = \frac{32.5K'(12,000)}{(12)(16.5'')^2} = 120$$

$$e = 1.33(.0023) = .0033 \leftarrow \text{GOVERNS}$$

$$(02) \quad e = .0033 \leftarrow \text{GOVERNS}$$



$$d = 20' - 3' \cdot .5 = 16.5''$$

YUCCA MOUNTAIN SITE CHARACTERIZATION PROJECT

PAGE NO. 8 OF 9

Civilian Radioactive Waste Management System

Management & Operating Contractor

CONTRACT NO. DE-AC01-91RW00134

SUBJECT: DESEL FUEL TANK FDN

WBS NO: 1.2.6.

DATE: 7-6-94

REV NO: _____

CALC NO: 54330000-0177-0200-0001

ORIGINATOR: M. GOMEZ

CHECKED BY: _____

CHECKED DATE: _____

$$A_s = .003(12)(16.5) = .59 \text{ in}^2/1$$

USE #7 @ 12" O.C. E.W.

$$A_s = 60 \text{ in}^2/1$$

11. CONCLUSIONS

The design shows that a concrete foundation that has minimum dimensions of 11' x 22' x 1'-8" thick, reinforced with #7 @ 12" o/c each way, is adequate to support the proposed Fuel Storage Tank and Fuel Pumps.

12. ATTACHMENTS

None.